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were the recession at
the berm of a berm
breakwater for
assumed design
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well as the functional
behaviour (wave
overtopping and
reflection), where often

allowable overtopping rates determine the crest height of the structure. They also gave guidance on how some geometrical aspects may influence Geometrical design of berm breakwaters v2 Lykke Andersen formula. Lykke Andersen (2006) arrived at the following dimensionless equation for the recession: (1)

$$R = c_1 \frac{h}{D} \left(\frac{h}{b} \right)^{c_2} \left(\frac{h}{s} \right)^{c_3} \left(\frac{h}{h_0} \right)^{c_4} \left(\frac{h}{h_b} \right)^{c_5} \left(\frac{h}{h_s} \right)^{c_6} \left(\frac{h}{h_0} \right)^{c_7} \left(\frac{h}{h_b} \right)^{c_8} \left(\frac{h}{h_s} \right)^{c_9} \left(\frac{h}{h_0} \right)^{c_{10}} \left(\frac{h}{h_b} \right)^{c_{11}} \left(\frac{h}{h_s} \right)^{c_{12}} \left(\frac{h}{h_0} \right)^{c_{13}} \left(\frac{h}{h_b} \right)^{c_{14}} \left(\frac{h}{h_s} \right)^{c_{15}} \left(\frac{h}{h_0} \right)^{c_{16}} \left(\frac{h}{h_b} \right)^{c_{17}} \left(\frac{h}{h_s} \right)^{c_{18}} \left(\frac{h}{h_0} \right)^{c_{19}} \left(\frac{h}{h_b} \right)^{c_{20}} \left(\frac{h}{h_s} \right)^{c_{21}} \left(\frac{h}{h_0} \right)^{c_{22}} \left(\frac{h}{h_b} \right)^{c_{23}} \left(\frac{h}{h_s} \right)^{c_{24}} \left(\frac{h}{h_0} \right)^{c_{25}} \left(\frac{h}{h_b} \right)^{c_{26}} \left(\frac{h}{h_s} \right)^{c_{27}} \left(\frac{h}{h_0} \right)^{c_{28}} \left(\frac{h}{h_b} \right)^{c_{29}} \left(\frac{h}{h_s} \right)^{c_{30}}$$

where:

- h : height of berm.
- D : design water level.
- b : berm width.
- s : seaward slope.
- h_0 : crest height.
- h_b : berm crest height.
- h_s : seaward slope height.

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Lykke Andersen
formula. Lykke
Andersen (2006)
arrived at the following
dimensionless equation
for the recession: (1)
 $Re\ c\ D\ n\ 50 = f\ h\ b \cdot 1$
 $+ c\ 1\ h - c\ 1\ h\ s\ h - h$
 $b \cdot f\ N \times f\ \beta \times f\ H\ 0 \times f$
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 $D\ n\ 50 \cdot h\ b - h$ where.
h b. height of berm.
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